

2D/3D beach morphology: the role of the wave potential stirring

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Funding: Spanish Government,
PUDEM project



**River Coastal and
Estuarine Morphodynamics**
October 4-7, 2005, Urbana, Illinois, USA



CONTENT

1. 2D vs. 3D morphology. Rhythmic morphology.
2. Sediment transport. Stirring function.
3. Bed evolution equation. Potential stirring.
4. Bed-surf instability. Shore-transverse bars.
5. Development of crescentic bars.
6. Conclusions

1. 2D vs. 3D MORPHOLGY. RHYTHMIC MORPHOLOGY.

‘2D’ morphology ($z_b(x)$)

**Sea bed topography
(and/or shoreline)
approximately uniform along
the coast**



‘3D’ morphology ($z_b(x,y)$)

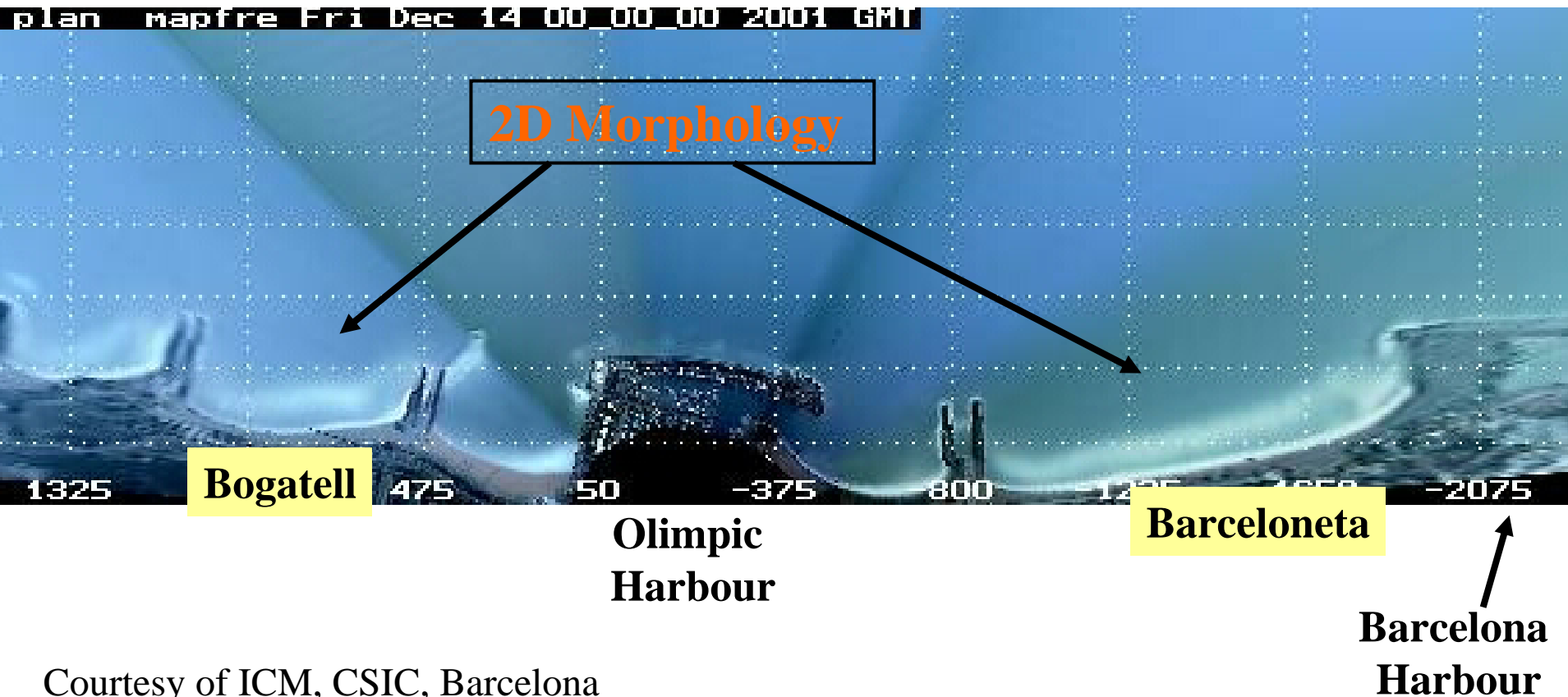
**Sea bed topography
(and/or shoreline)
with alongshore gradients**



Australian beaches

1. 2D vs. 3D MORPHOLOGY. RHYTHMIC MORPHOLOGY.

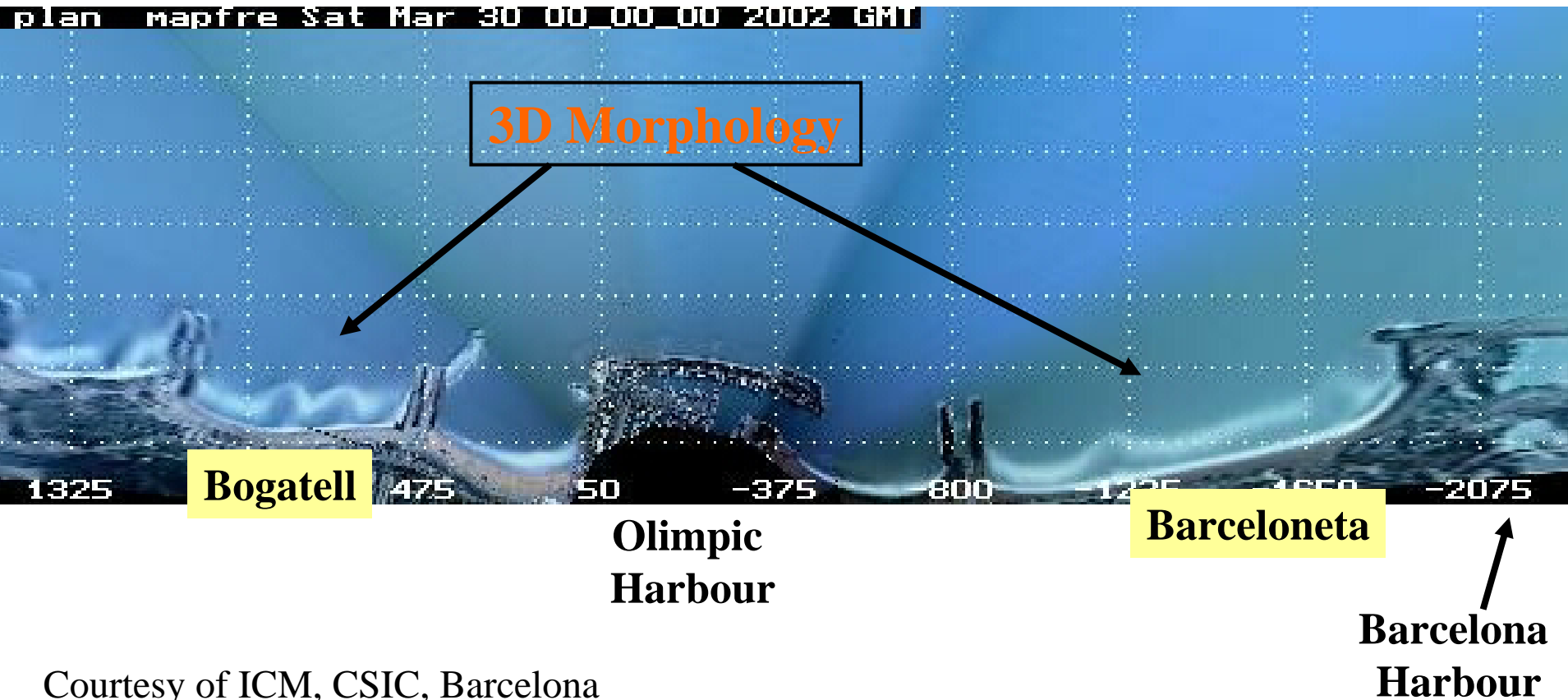
A beach may change from 2D to 3D and viceversa



Courtesy of ICM, CSIC, Barcelona

1. 2D vs. 3D MORPHOLOGY. RHYTHMIC MORPHOLOGY.

A beach may change from 2D to 3D and viceversa



1. 2D vs. 3D MORPHOLOGY. RHYTHMIC MORPHOLOGY.

Basic field knowledge

Classification of wave conditions with respect to sediment size
(which if time allows give rise to a particular beach equilibrium state)
(Wright y Short, 1984; Sunamura, 1988; Lippmann y Holman, 1990)

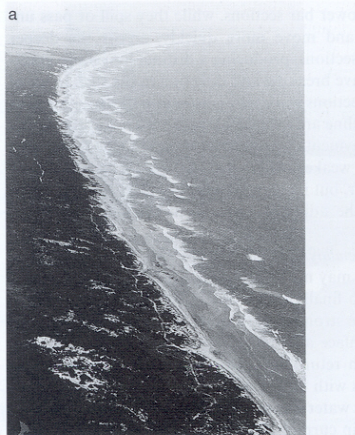
Dissipative conditions

All the incident wave energy is dissipated



Fine sand, large waves
wave period relatively short

Intermediate conditions



**3D Morphology
(in the surf zone)**

Reflective conditions

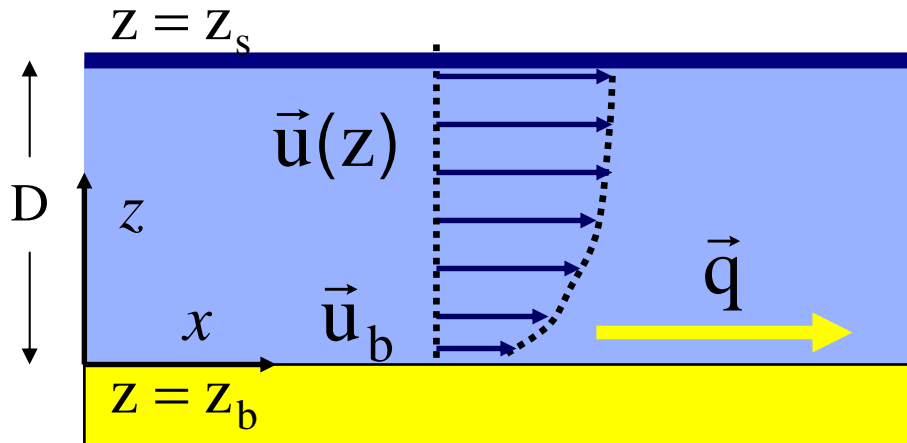
A significant fraction of wave energy is reflected (up to ~ 30 %)



Coarse sand, small waves
wave period relatively long

2. SEDIMENT TRANSPORT. THE STIRRING FUNCTION

Bedload transport by a current



Vertically averaged sediment flux
 = total volume crossing the
 horizontal length unit per time unit
 = $\text{m}^3 / \text{m} \times \text{s} = \text{m}^2 \text{s}^{-1}$

Bagnold, 1963

$$\vec{q} = r \left(u_b^2 - u_{cri}^2 \right) \vec{u}_b$$

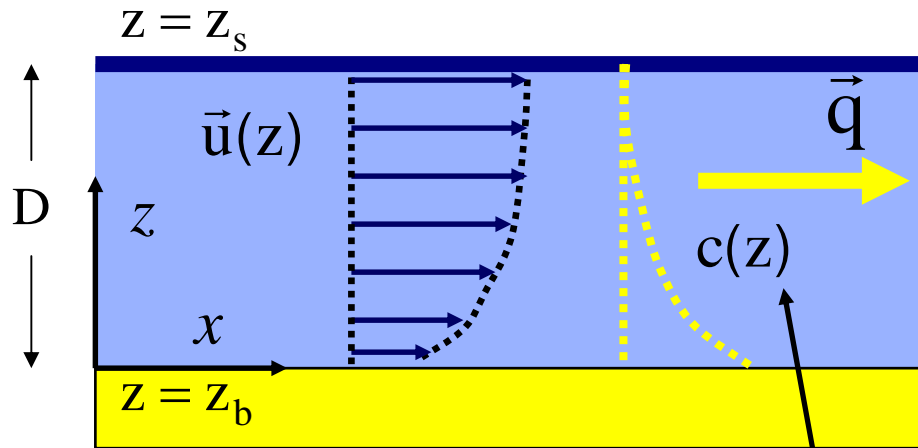
$$\vec{v} = \langle \vec{u} \rangle = \frac{1}{D} \int_{z_b}^{z_s} \vec{u}(z) dz \longrightarrow \vec{u}_b = \mu \vec{v}$$

$$\vec{q} = \hat{r} \left(v^2 - v_{cri}^2 \right) \vec{v}$$

$$\vec{q} = \alpha(v) \vec{v}$$

2. SEDIMENT TRANSPORT. THE STIRRING FUNCTION

Suspended load transport by a current



Suspended load concentration (m^3/m^3)

$$\vec{v} = \langle \vec{u} \rangle$$

$$\vec{q} = \int_{z_b}^{z_s} c(z) \vec{u}(z) dz = \langle c \rangle \langle \vec{u} \rangle \int_{z_b}^{z_s} g(z) f(z) dz = \mu D \langle c \rangle \langle \vec{u} \rangle = \alpha \vec{v}$$

$$\begin{aligned} \vec{u}(z) &= f(z) \langle \vec{u} \rangle \\ c(z) &= g(z) \langle c \rangle \end{aligned}$$

Stirring function or
equivalent depth integrated
concentration $\alpha = \mu D \langle c \rangle$

$$\vec{q} = \alpha(v) \vec{v}$$

2. SEDIMENT TRANSPORT. THE STIRRING FUNCTION

However, many complications arise:

- ❑ Vertical profiles vary in space and time:
 - in particular, $c(x,y,z,t)$ has its own dynamics
- ❑ Current + wave oscillatory flow
 - very often in different directions

Still, there are relatively simple formulations that work reasonably well in the nearshore with waves + currents.

For example:

- Bailard
- Soulsby-Van Rijn

It makes sense to assume:

$$\vec{q} = \alpha(v, u_o, D, \dots) \vec{v}$$

u_o = bed wave orbital velocity

3. BED EVOLUTION EQUATION. POTENTIAL STIRRING

sediment conservation $\frac{\partial z_b}{\partial t} + \nabla \cdot (\alpha \vec{v}) = 0$

water conservation $\frac{\partial D}{\partial t} + \nabla \cdot (D \vec{v}) = 0$

$$\frac{\partial z_b}{\partial t} + \nabla \cdot \left(\frac{\alpha}{D} D \vec{v} \right) = 0 \longrightarrow \frac{\partial z_b}{\partial t} + D \vec{v} \cdot \nabla \left(\frac{\alpha}{D} \right) + \frac{\alpha}{D} \nabla \cdot (D \vec{v}) = 0$$

$$\frac{\partial z_b}{\partial t} + D \vec{v} \cdot \nabla \left(\frac{\alpha}{D} \right) - \frac{\alpha}{D} \frac{\partial D}{\partial t} = 0$$

$$D = z_s - z_b$$

$$\frac{\alpha}{D} \ll 1, \quad \frac{\partial z_s}{\partial t} \approx \frac{\partial z_b}{\partial t}$$

$$\left(1 + \frac{\alpha}{D} \right) \frac{\partial z_b}{\partial t} + D \vec{v} \cdot \nabla \left(\frac{\alpha}{D} \right) = \frac{\alpha}{D} \frac{\partial z_s}{\partial t}$$

$$\frac{\partial z_b}{\partial t} = -D \vec{v} \cdot \nabla \left(\frac{\alpha}{D} \right)$$

3. BED EVOLUTION EQUATION. POTENTIAL STIRRING

Bed evolution equation BEE

$$\frac{\partial z_b}{\partial t} = -D \vec{v} \cdot \nabla \left(\frac{\alpha}{D} \right)$$

$$\frac{\alpha}{D} = \text{'potential stirring'}$$

or depth averaged concentration $\sim \langle c \rangle$

CONSEQUENCE:

Accretion condition $(\frac{\partial z_b}{\partial t} > 0)$:

$$\vec{v} \cdot \nabla \left(\frac{\alpha}{D} \right) < 0$$

Current *against* the gradient in potential stirring

Erosion condition $(\frac{\partial z_b}{\partial t} < 0)$:

$$\vec{v} \cdot \nabla \left(\frac{\alpha}{D} \right) > 0$$

Current *with* the gradient in potential stirring

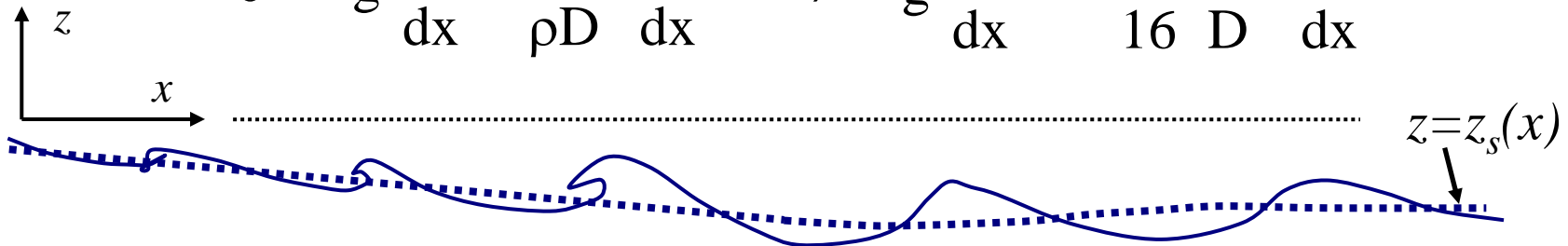
4. 'BED-SURF' INSTABILITY. SHORE-TRANSVERSE BARS.

Shore-normal wave incidence
No alongshore gradients

Mean hydrodynamics:

- water conservation $\Rightarrow v_x = 0$
- alongshore momentum balance $\Rightarrow v_y = 0$
- cross-shore momentum balance :

$$0 = -g \frac{dz_s}{dx} - \frac{1}{\rho D} \frac{dS_{xx}}{dx} \Rightarrow g \frac{dz_s}{dx} = -\frac{3}{16} \frac{gH}{D} \frac{dH}{dx}$$



surf zone

$$\frac{dH}{dx} > 0 \Rightarrow \frac{dz_s}{dx} < 0$$

shoaling zone

$$\frac{dH}{dx} < 0 \Rightarrow \frac{dz_s}{dx} > 0$$

4. 'BED-SURF' INSTABILITY. SHORE-TRANSVERSE BARS.

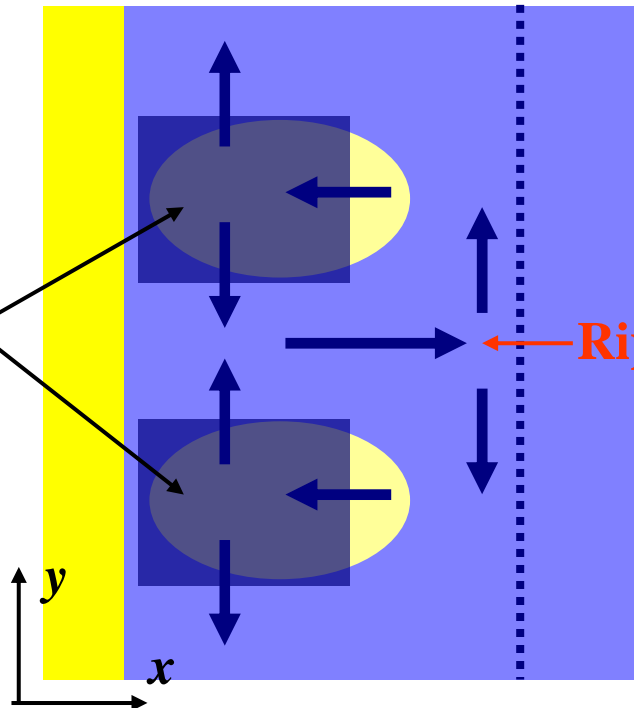
Shore-normal wave incidence With alongshore gradients

Mean hydrodynamics:

By assuming the same
momentum balance in x:

$$\sigma_g \frac{dz_s}{dx} = -\frac{3}{16} \frac{gH}{D} \frac{dH}{dx}$$

Larger set-up in
the mean water
level



Rip current

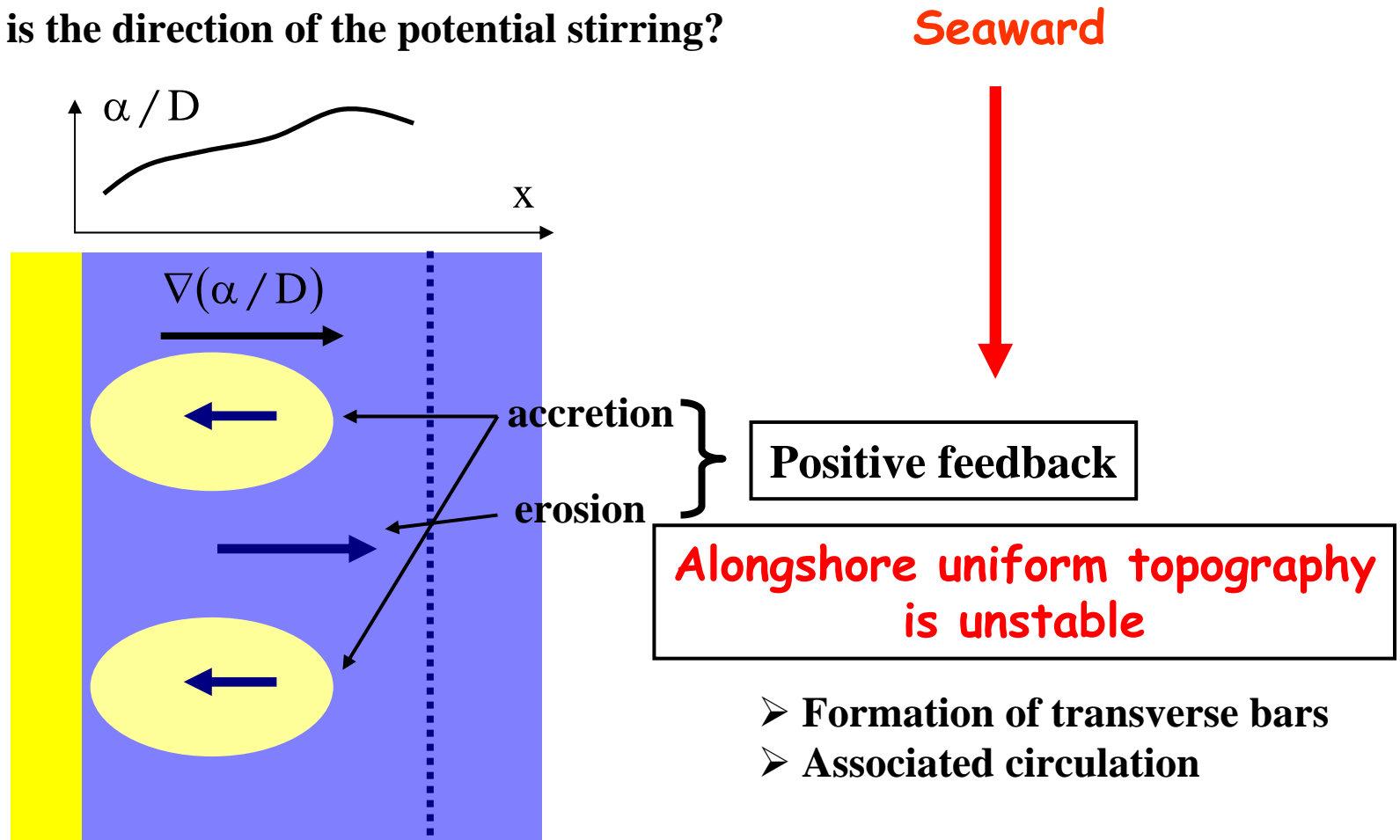
There is no
possible equilibrium
without currents.

4. 'BED-SURF' INSTABILITY. SHORE-TRANSVERSE BARS.

Shore-normal wave incidence

Morphodynamic instability:

Wich is the direction of the potential stirring?

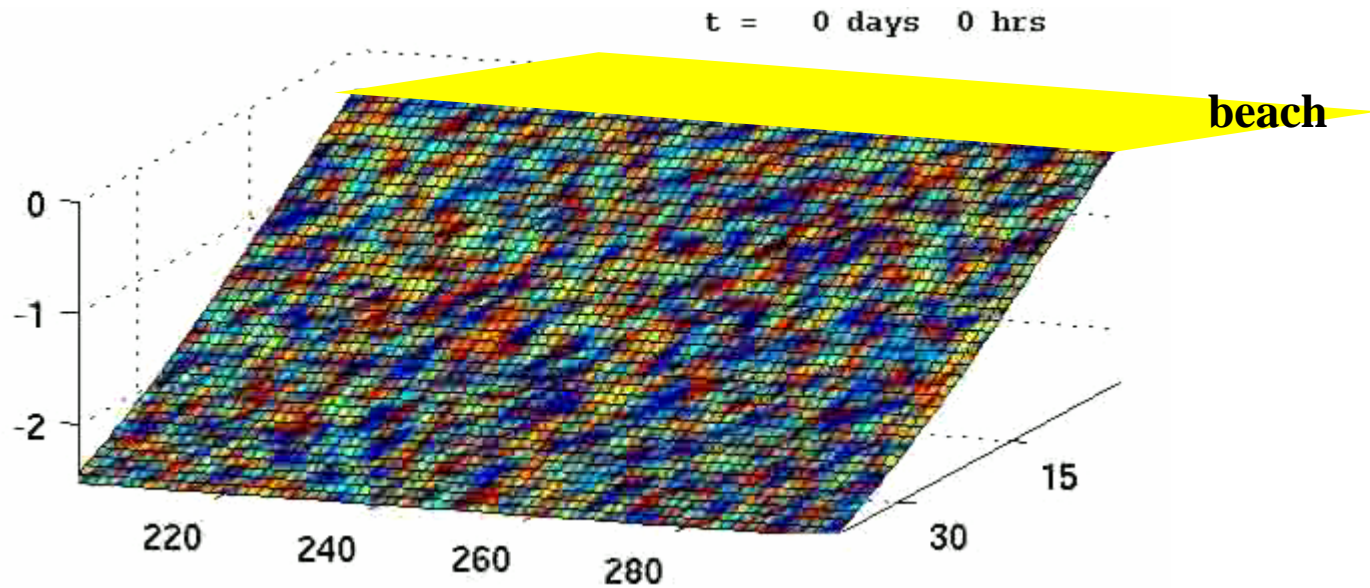


4. 'BED-SURF' INSTABILITY. SHORE-TRANSVERSE BARS.

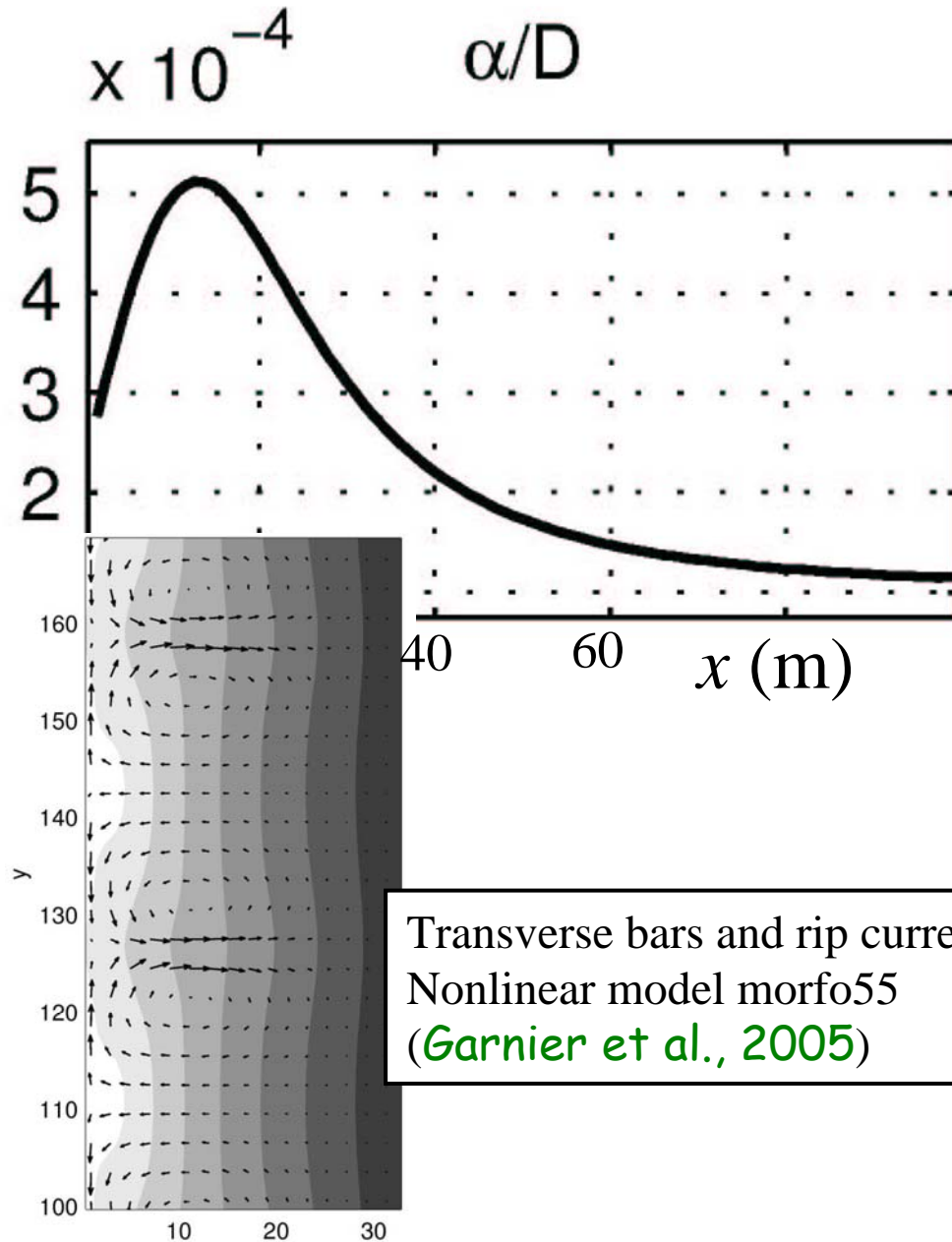
Numerical simulation of the formation of transverse bars from initial small bed perturbations.

**Shore-normal incident waves with $H=1$ m
morfo55 model.**

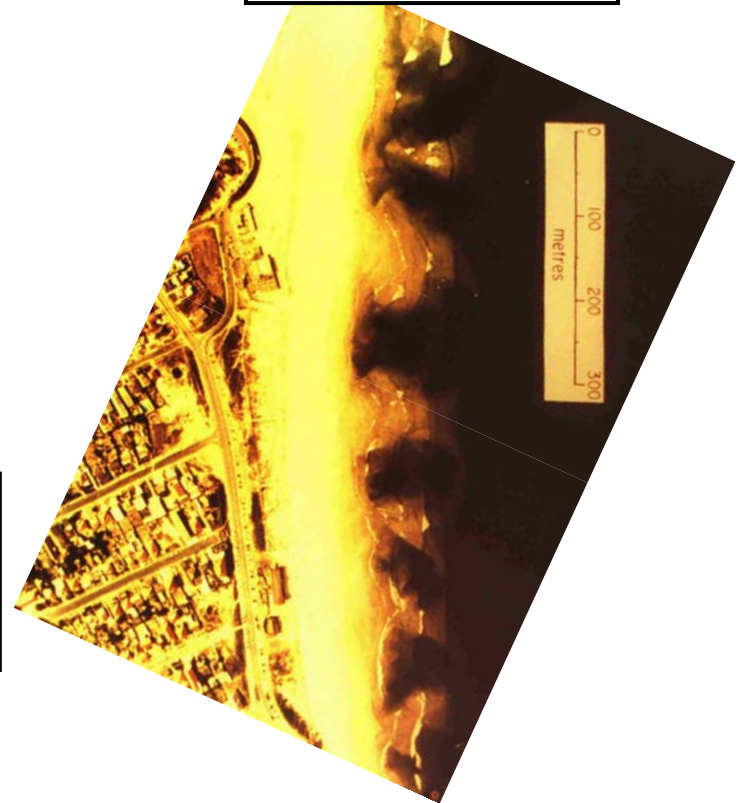
(Garnier et al., 2005)



4. 'BED-SURF' INSTABILITY. SHORE-TRANSVERSE BARS.



Aerial picture of transverse bars



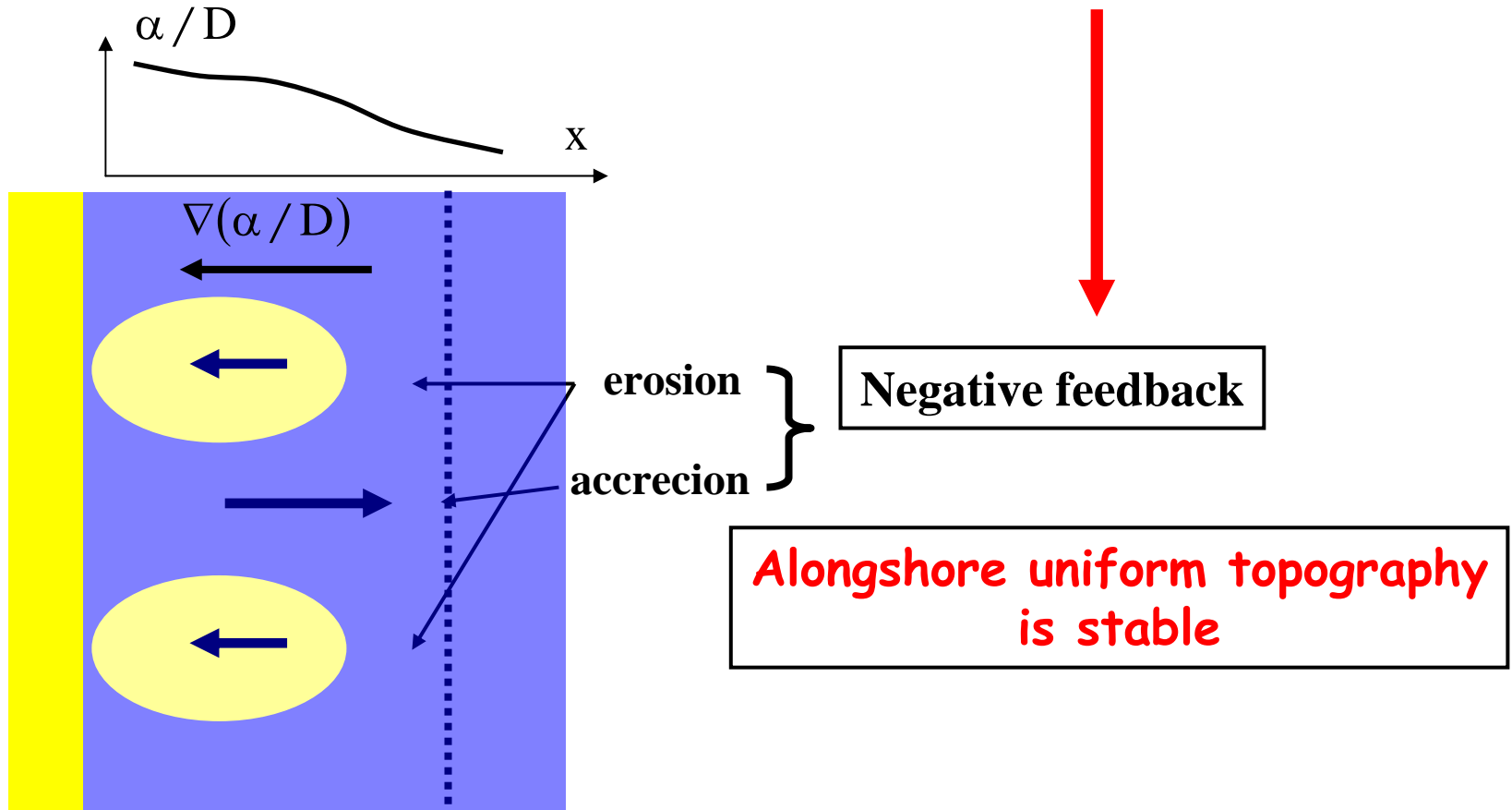
4. 'BED-SURF' INSTABILITY. SHORE-TRANSVERSE BARS.

Shore-normal wave incidence

Morphodynamic instability:

Wich is the direction of the potential stirring?

Shoreward



4. 'BED-SURF' INSTABILITY. SHORE-TRANSVERSE BARS.

Therefore:

- seaward gradient in potential stirring in the surf zone
 - INSTABILITY
- shoreward gradient
 - STABILITY

BUT what determines this gradient?

$$\vec{q} = \alpha(v, u_o) \vec{v} \approx \alpha(u_o) \vec{v} = \mu u_o^n \vec{v}$$

initially, $v \ll u_o$

Wave orbital velocity $u_o \approx \frac{1}{2} \gamma_b \sqrt{gD}$

}

$\frac{\alpha}{D} \propto D^{(n/2)-1}$

4. 'BED-SURF' INSTABILITY. SHORE-TRANSVERSE BARS.

$$\frac{\alpha}{D} \propto D^{(n/2)-1}$$

➤ bedload, $n = 2 \Rightarrow \frac{\alpha}{D} \approx \text{cte.} \Rightarrow \text{stability}$

➤ suspended load, $n > 2 \Rightarrow \frac{\alpha}{D}$ Seaward increasing $\Rightarrow \text{instability}$

bedload: coarse sand, small waves \rightarrow reflective conditions

Suspended load: fine sand, large waves \rightarrow dissipative or intermediate conditions

Observations show that 3D morphology occurs in intermediate conditions

BUT what happens for dissipative conditions?

4. 'BED-SURF' INSTABILITY. SHORE-TRANSVERSE BARS.

Infragravity waves

In addition to ordinary wind or swell waves with $T \sim 1-20$ s
There are low frequency waves with $T \sim 20$ s - O(1 min.))

Because of their low frequency these waves do not break at the shoreline \Rightarrow
Their amplitude is maximum at the shoreline (shoaling).

Wind/swell wave orbital velocity ' + ' infragravity wave orbital velocity
shoreward decreasing *shoreward increasing*

$\alpha \approx cte. \Rightarrow \frac{\alpha}{D}$ seaward decreasing \Rightarrow STABILITY

The ratio (energy in the infragravity band / wind/swell wave energy)
is maximum for dissipative conditions \Rightarrow

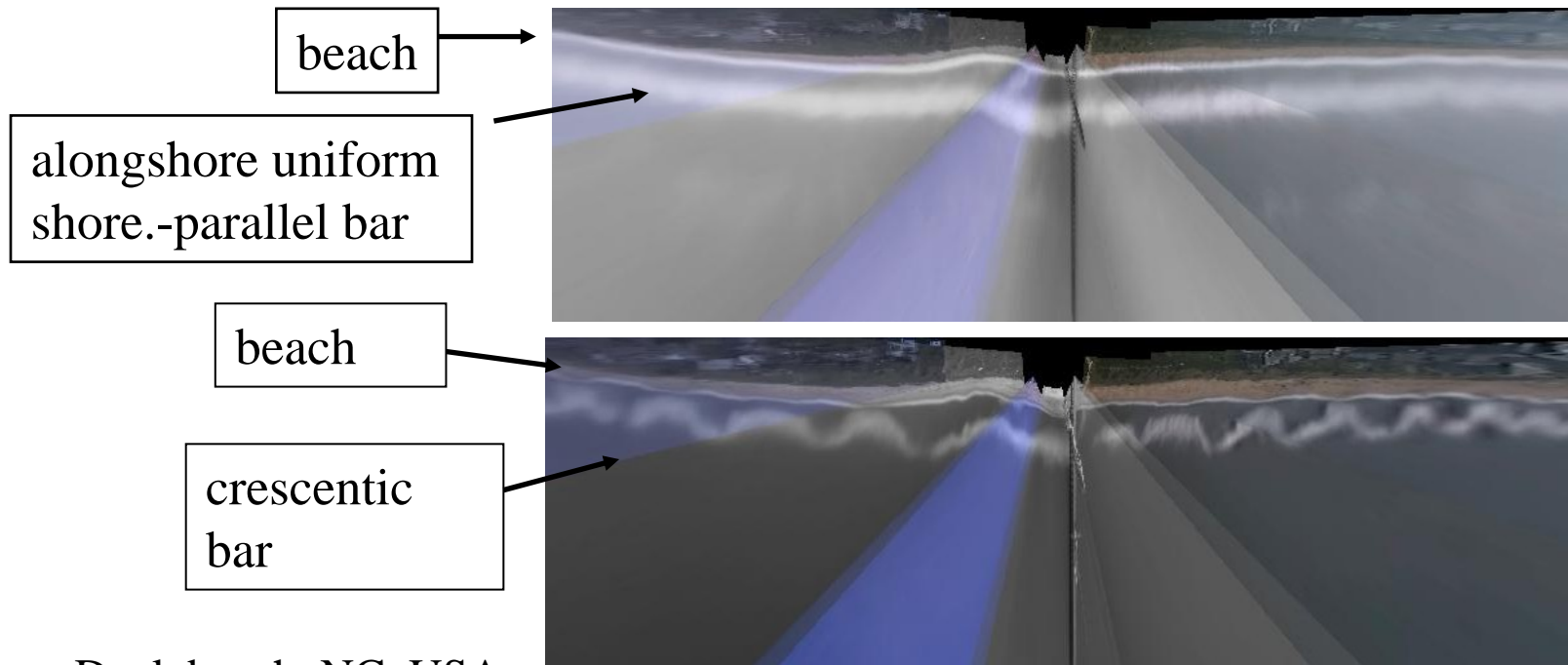
Dissipative conditions \Rightarrow

Morphodynamic stability under alongshore non-uniform perturbations

5. DEVELOPMENT OF CRESCENTIC BARS.

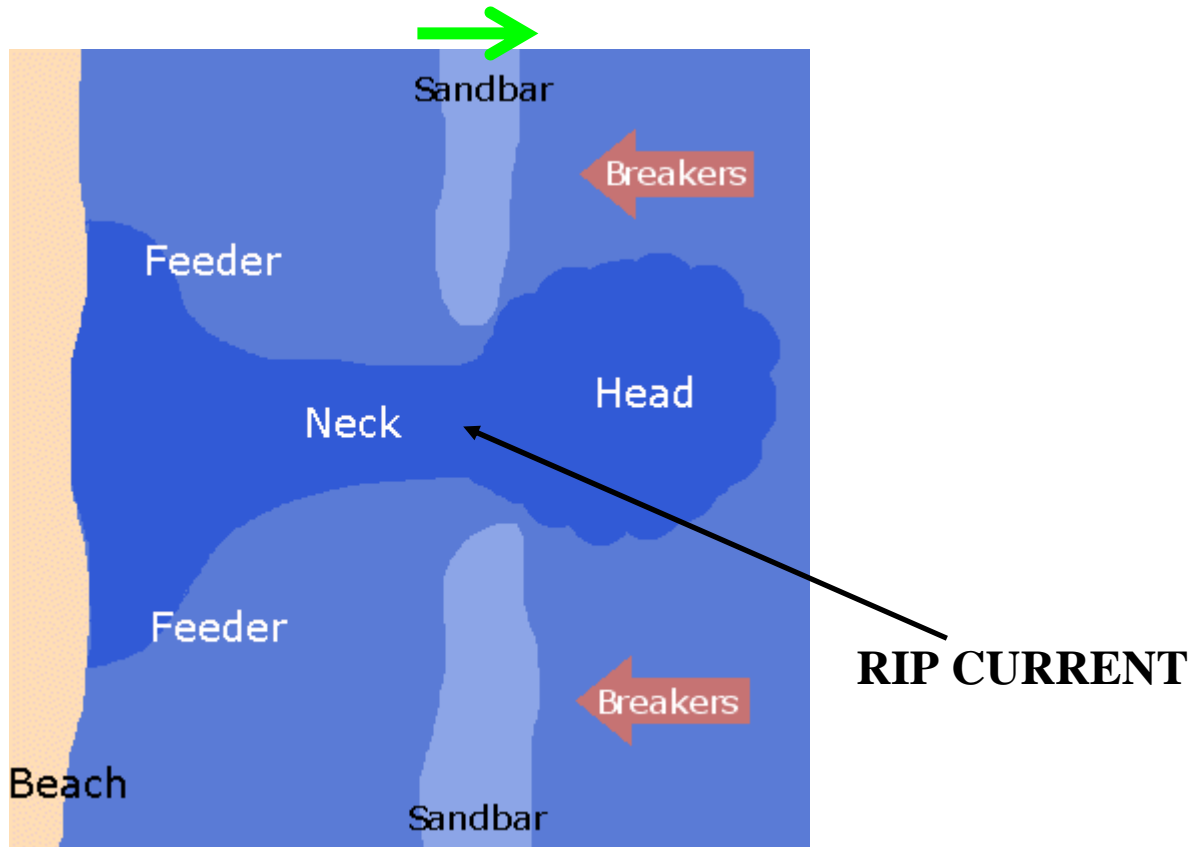
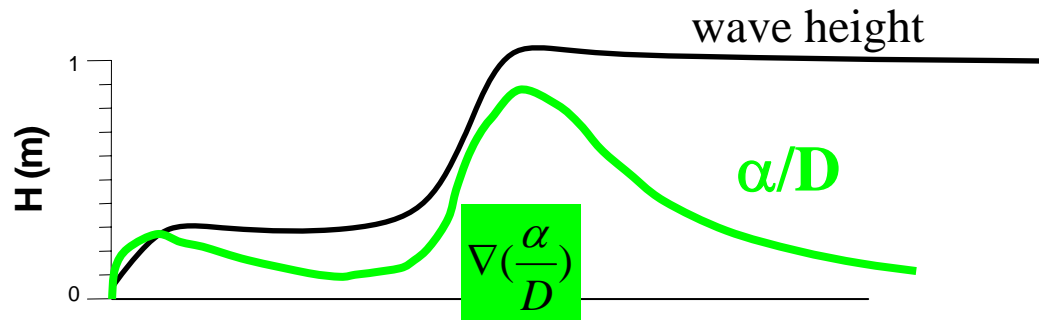
Crescentic bars are the most common (at least best known) type of rhythmic topography.

Can bed-surf instability explain why a shore-parallel bar becomes crescentic?



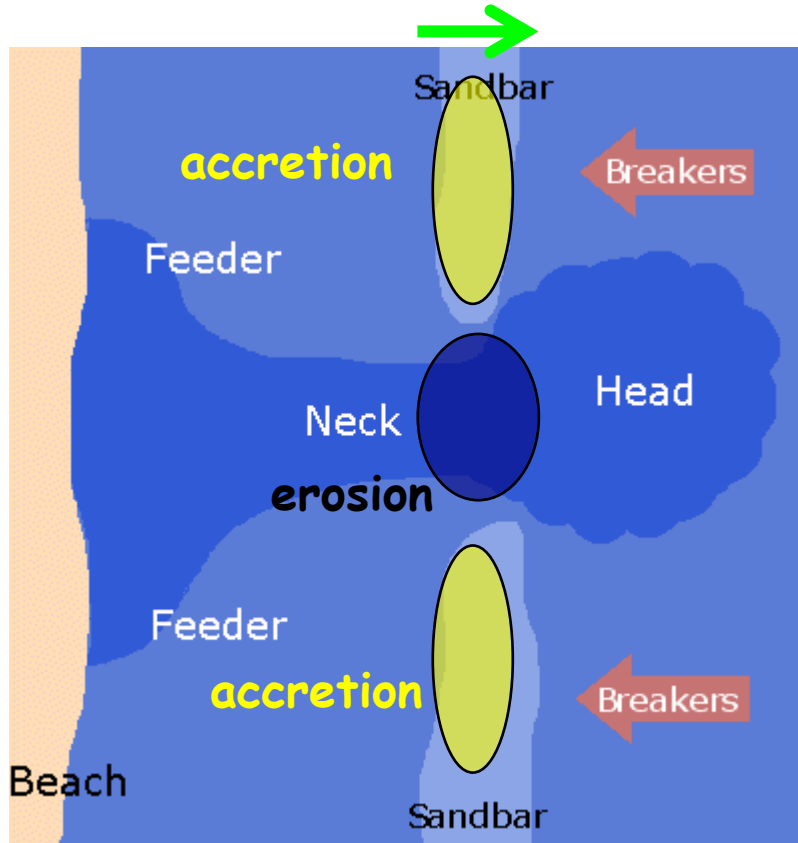
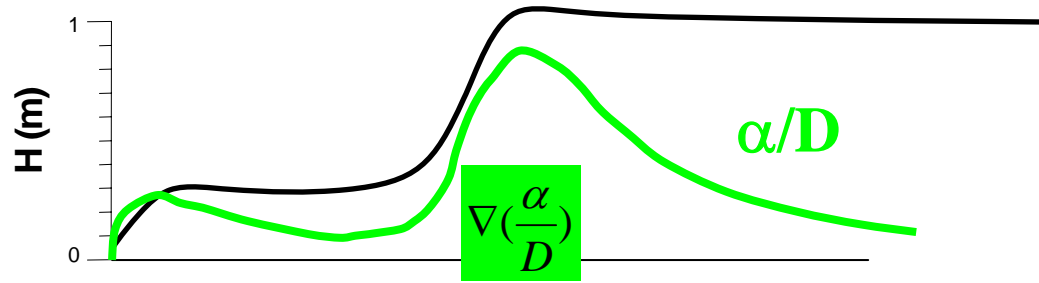
Duck beach, NC, USA

5. DEVELOPMENT OF CRESCENTIC BARS.



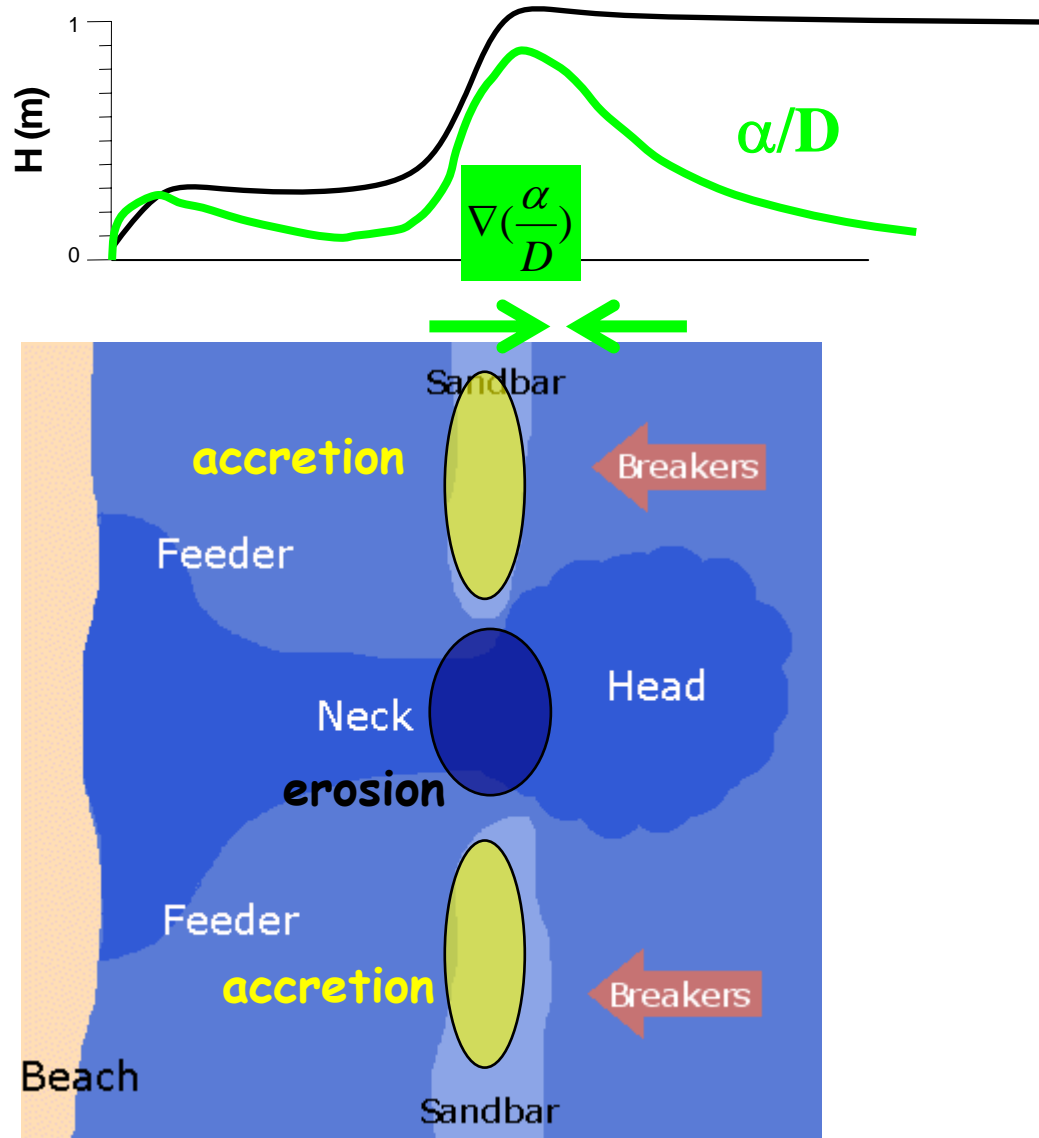
Courtesy
of NOAA

5. DEVELOPMENT OF CRESCENTIC BARS.

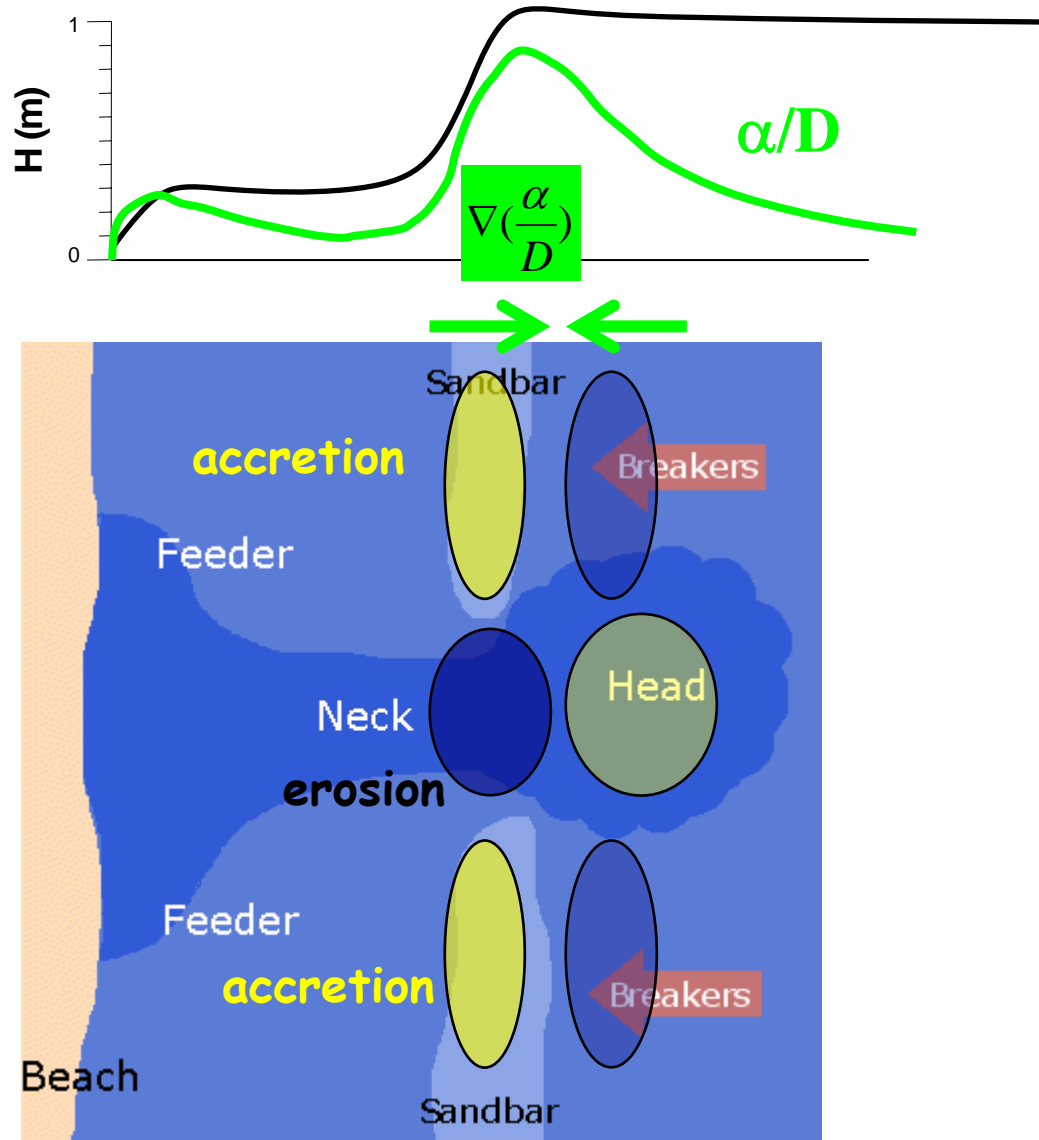


**Positive
feedback**

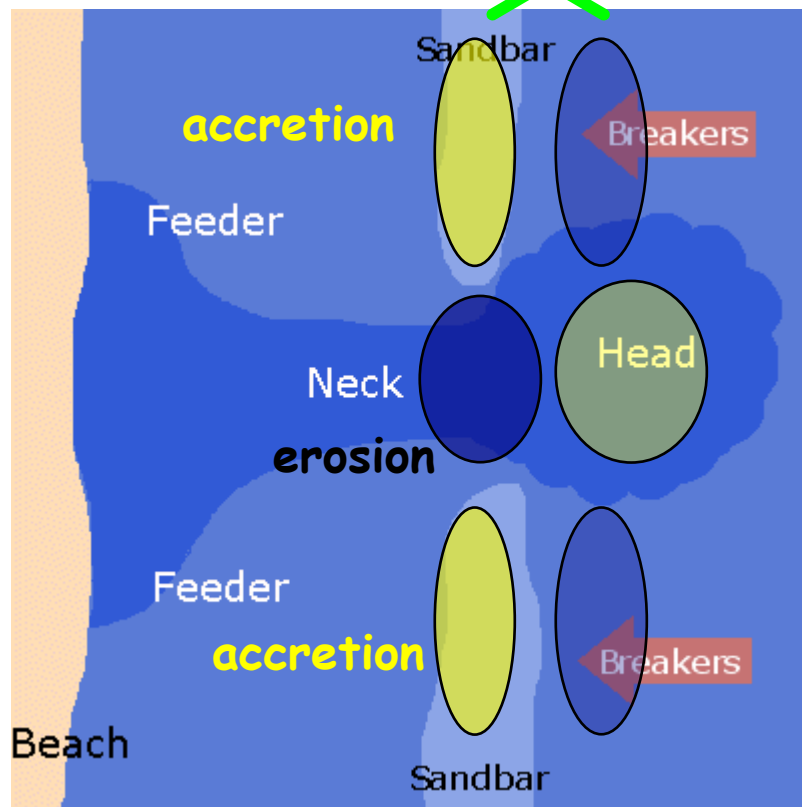
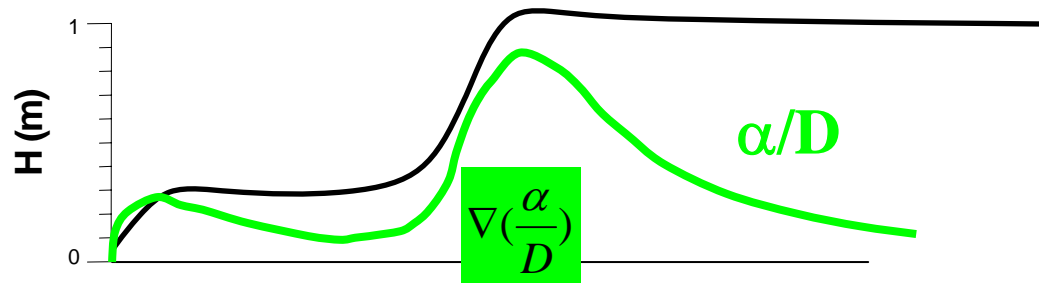
5. DEVELOPMENT OF CRESCENTIC BARS.



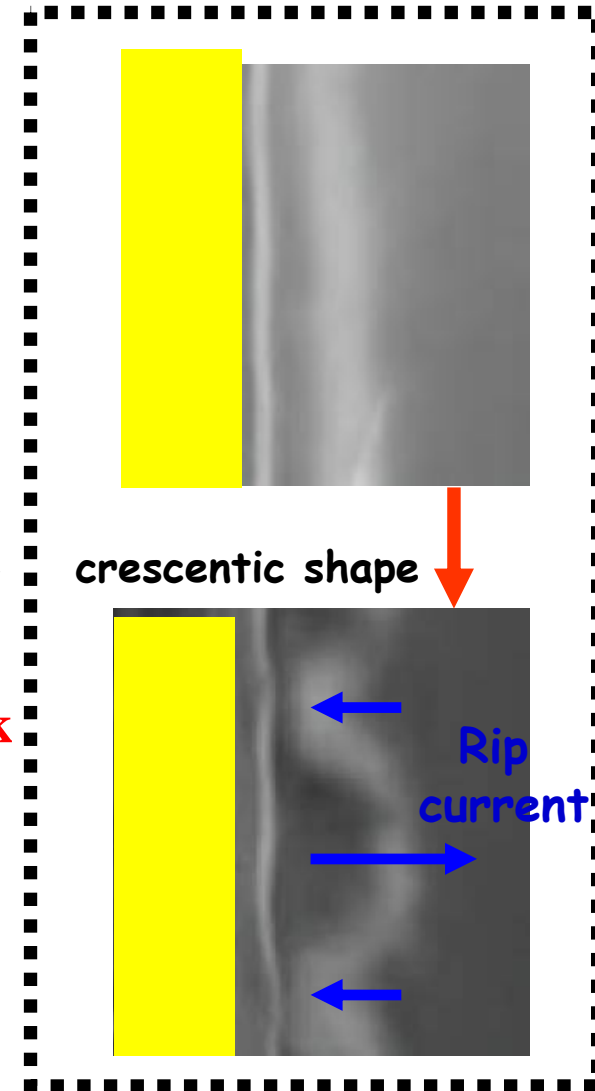
5. DEVELOPMENT OF CRESCENTIC BARS.



5. DEVELOPMENT OF CRESCENTIC BARS.



Positive feedback



6. CONCLUSIONS.

- ❖ The potential stirring or depth averaged equivalent concentration is a promising tool to understand and predict
 - the stability/instability of 2D morphology
 - the emerging morphology when 2D morphology is unstable

- ❖ Field experiments are needed to measure depth averaged equivalent concentration profiles and check against observed morphodynamics

- ❖ Limitations: more complex sediment transport processes:
 - anisotropy
 - transport driven by waves without mean current
 - dynamics of sediment concentration, space and time lags, ...
 - dynamics of the vertical structure (undertow, ...)